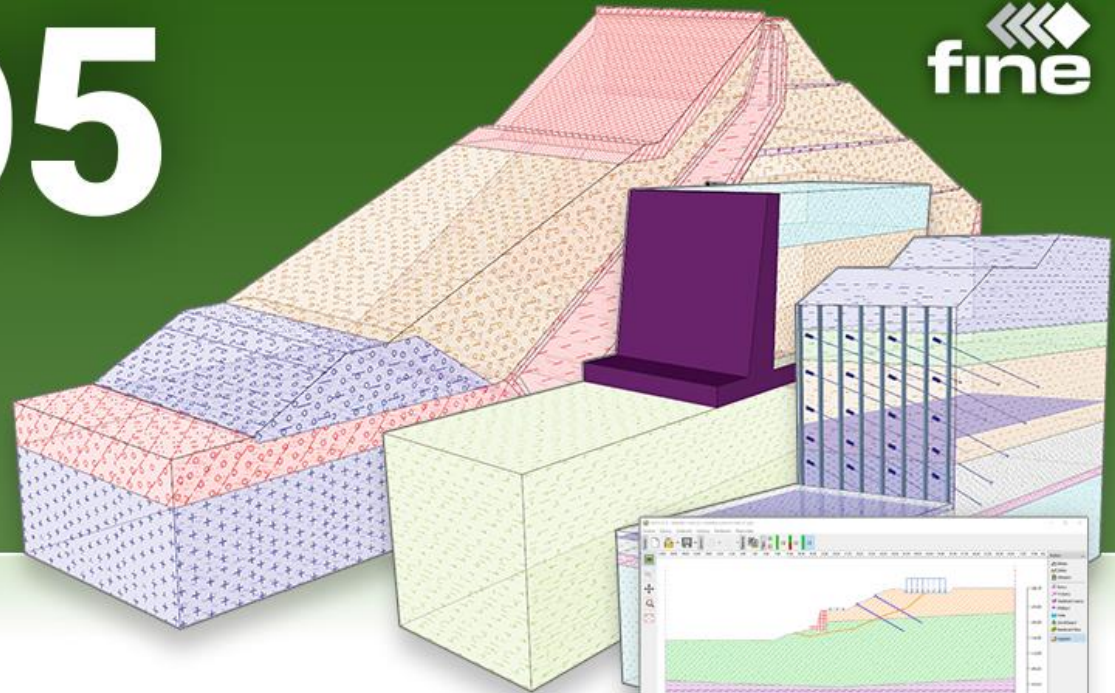




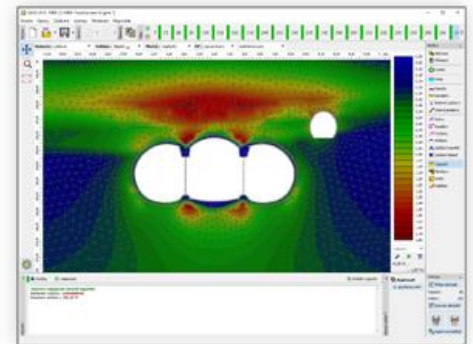
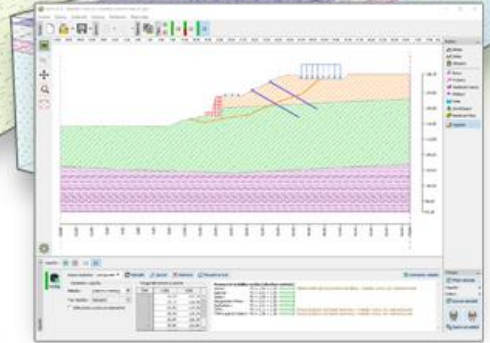
# GEO5



## Geotechnical Software



SOFTWARE  
GEOTECHNICAL



# FEM – Fundamentals

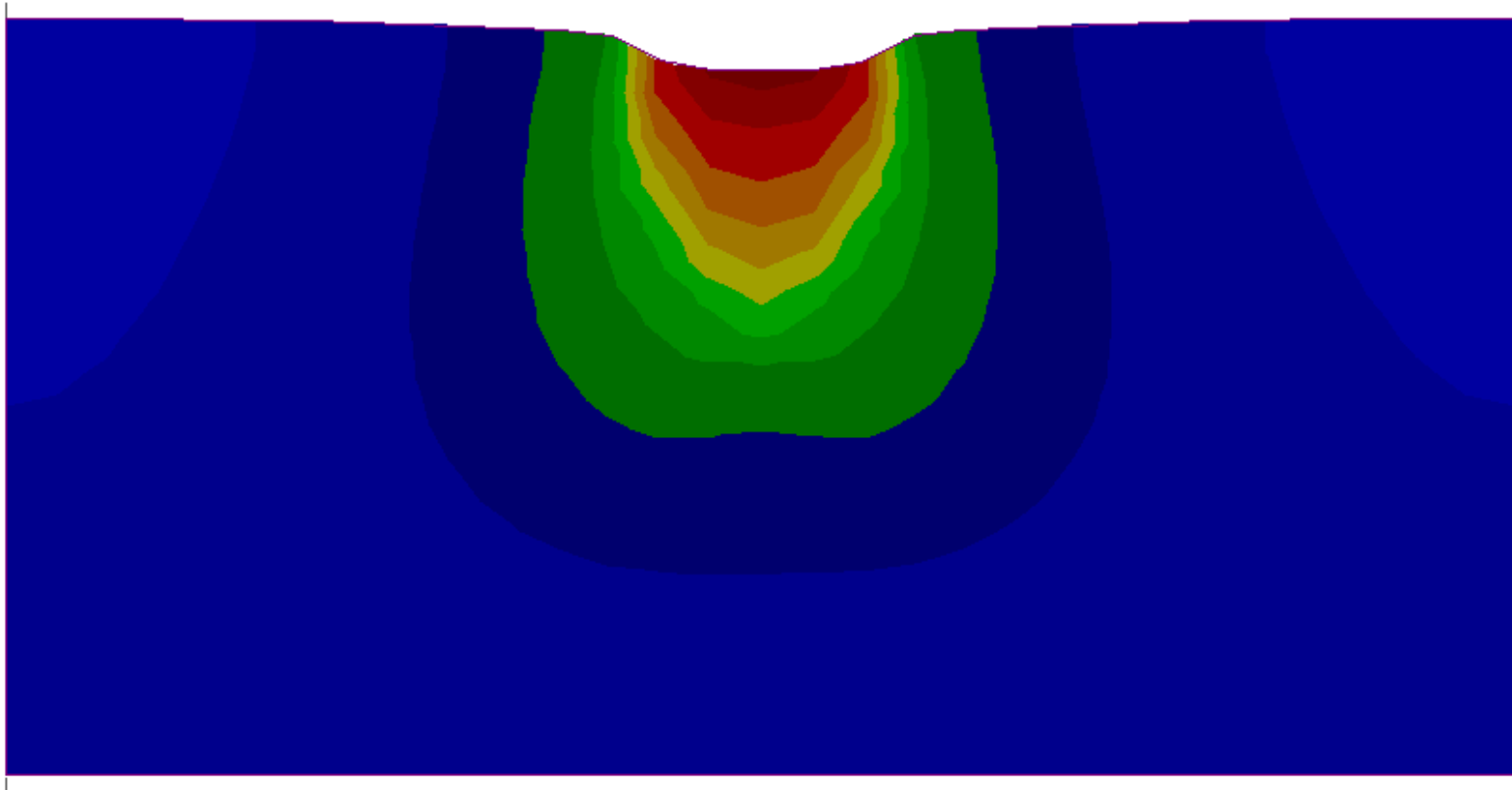
Tomáš Janda

[www.finesoftware.eu](http://www.finesoftware.eu)



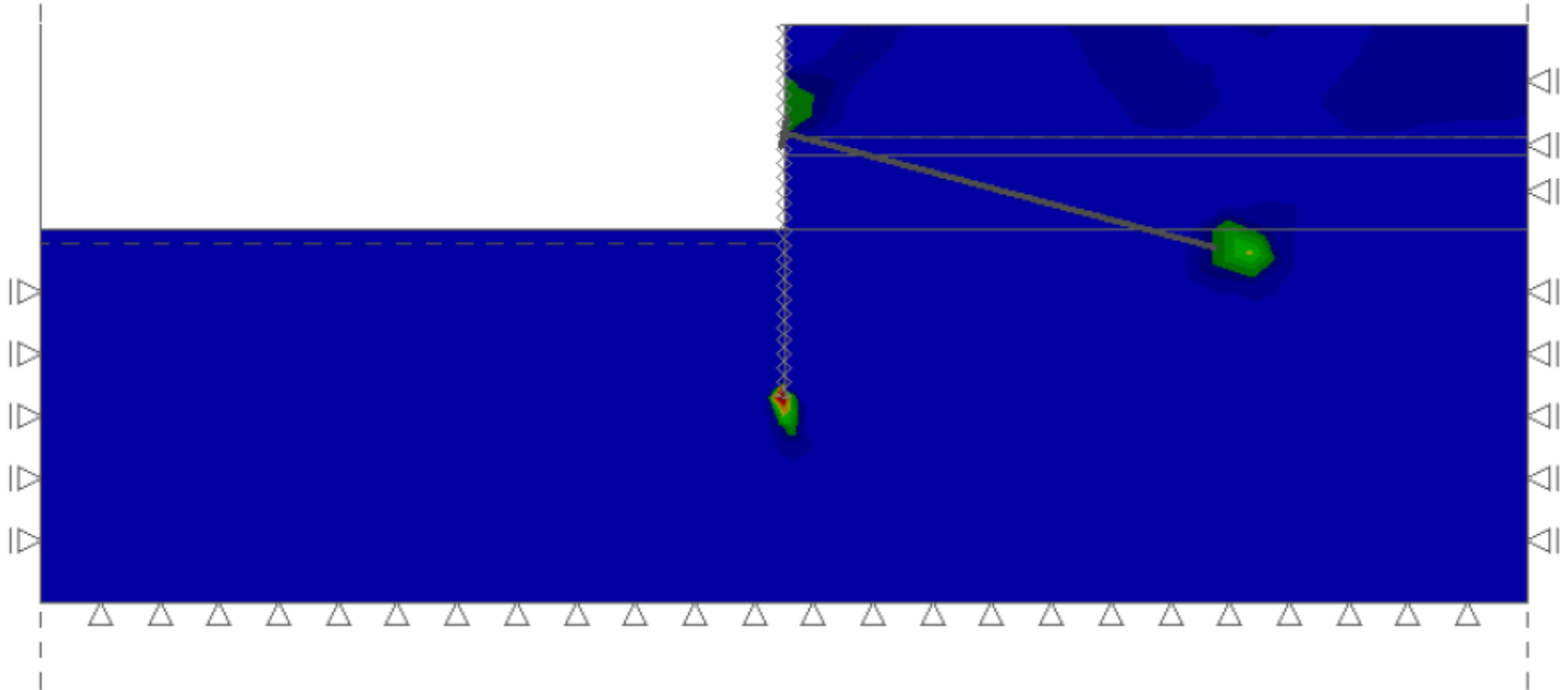
# Motivation

- Settlement



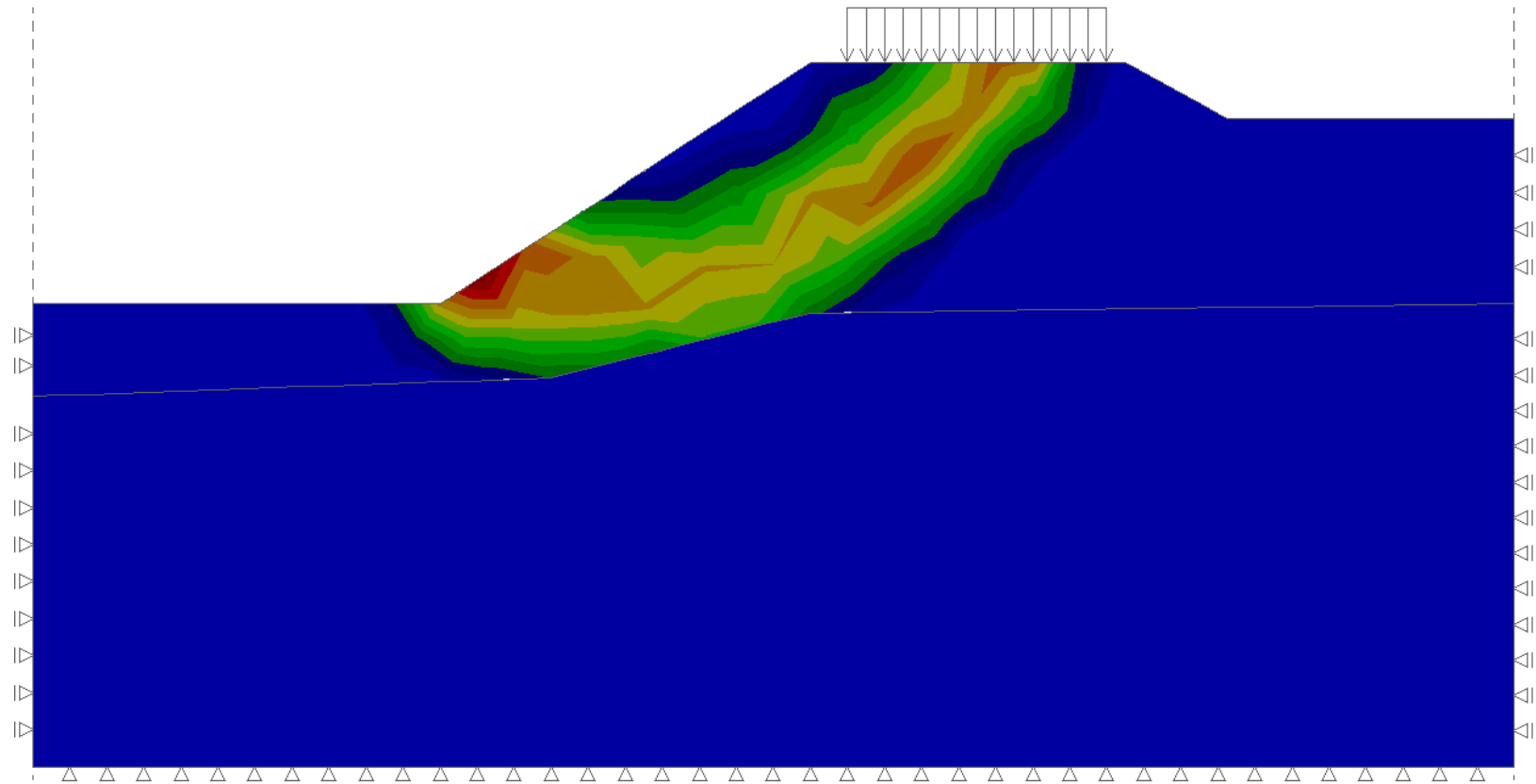
# Motivation

- Retaining walls



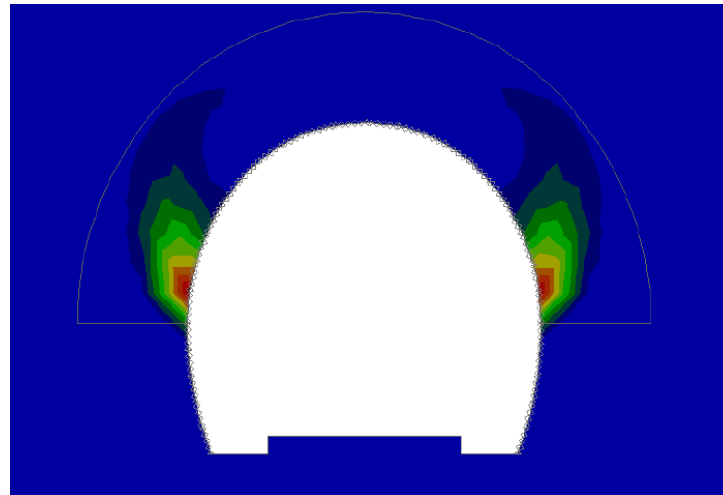
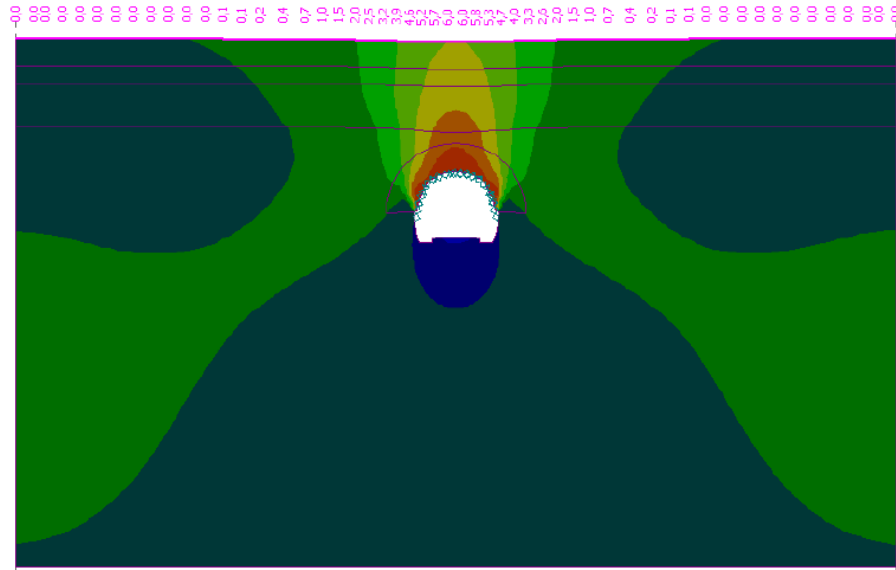
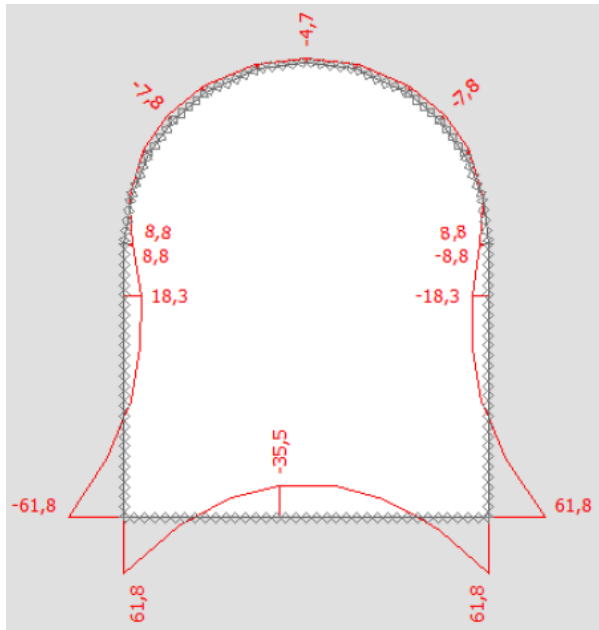
# Motivation

- Slope stability



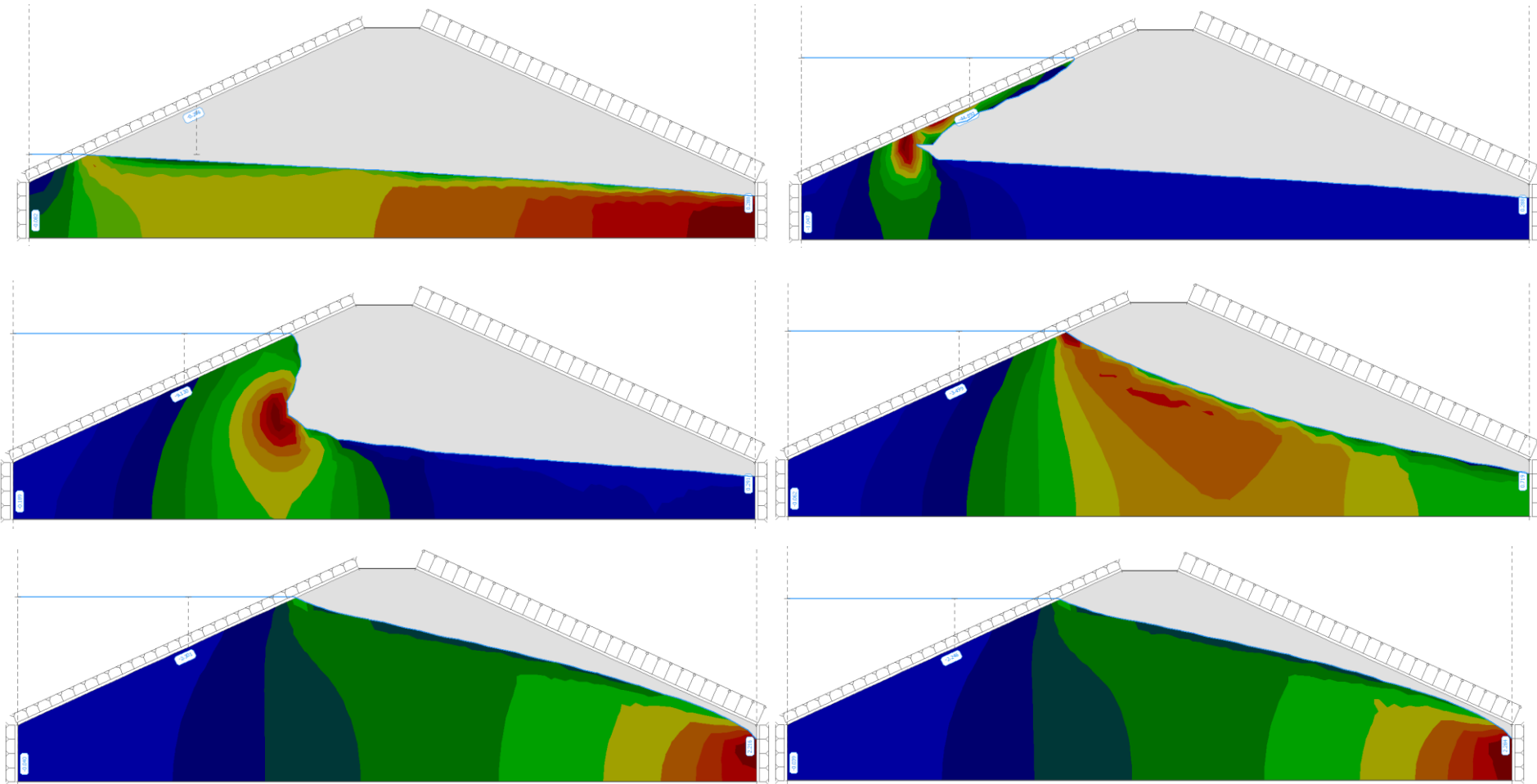
# Motivation

- Tunnels



# Motivation

- Transient water flow through an earth dam

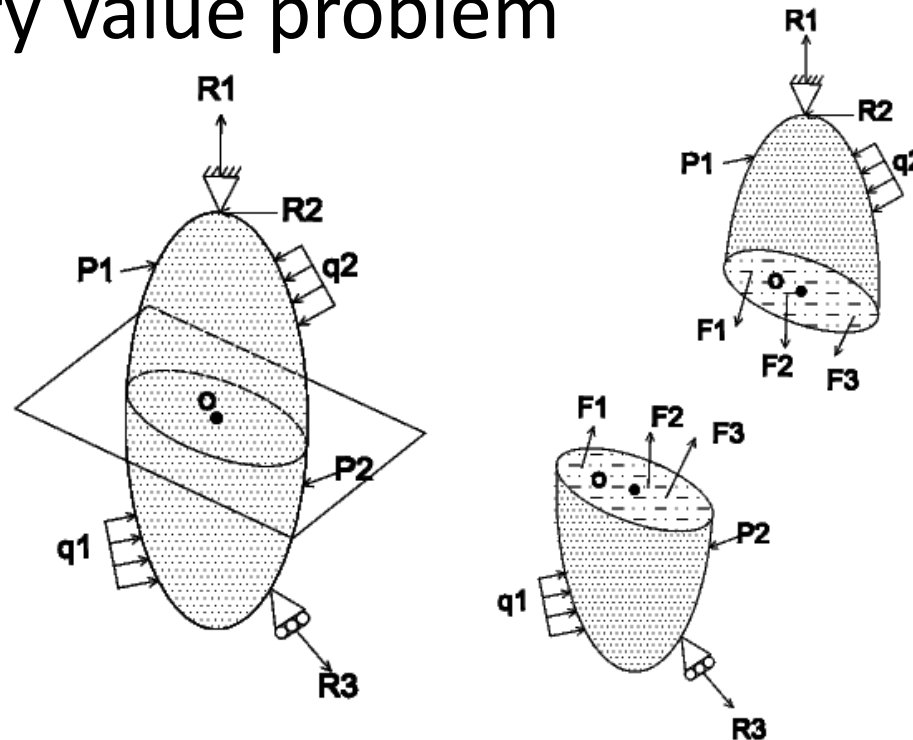


# Outline

- 2 slides of theory
- Advantages vs. limitations
- **Essential terms** and principles
- Universal rule of FEM modeling
- FE mesh
- Stability

# GEO 5 FEM - Theory

- Finite Element Method
  - Gives approximate solution to boundary value problem for partial differential equations.
- Boundary value problem





# GEO 5 FEM - Theory

- Partial differential equation (PDE)
- Fulfilled at every point
- Various problems → various types of PDE
  - Mechanical
    - Static equations:  $\partial\sigma + X = 0$
    - Stress strain relationship:  $\sigma = D(\varepsilon - \varepsilon_0)$
    - Geometric equations:  $\varepsilon = \partial^T u$
  - Water flow
    - Continuity equations:  $n \frac{\partial S}{\partial t} + \nabla[nSv^w] = m$
    - Darcy law:  $nSv^w = -\frac{K_r}{\gamma_w} (\nabla p - \gamma_w i_g)$

# FEM vs. Design codes

FEM analysis approximate the *physics* behind the behavior of the structure

## Advantages

- Arbitrary geometry
- Multiple phenomena
- Very flexible models
- Helps you to understand the behavior
- Universal principles

## Limitations

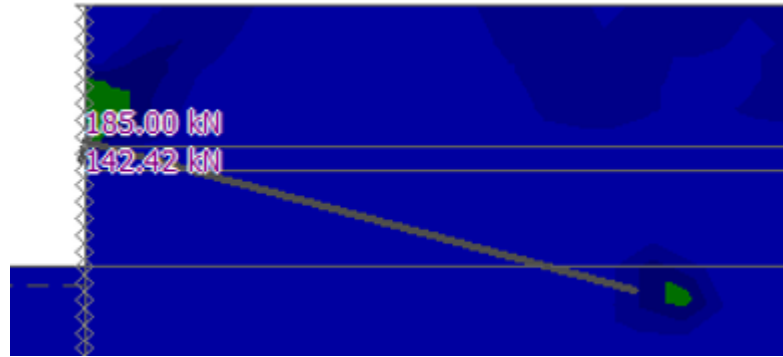
- No safety coefficients
- No design/characteristic values
- No design standards
- Not always on safe side
- Simplifications have to be made

# Essential terminology

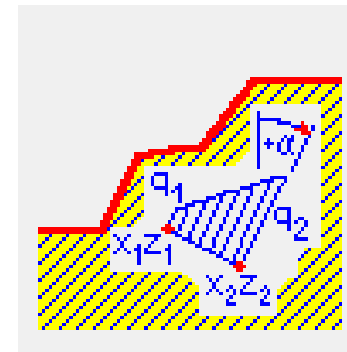
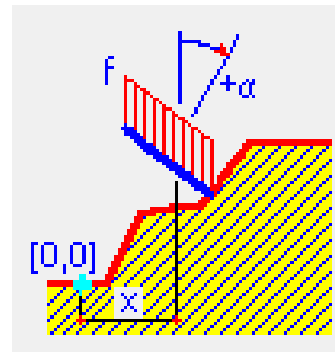
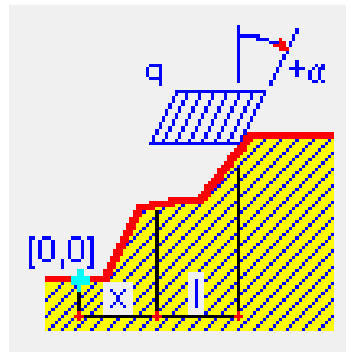
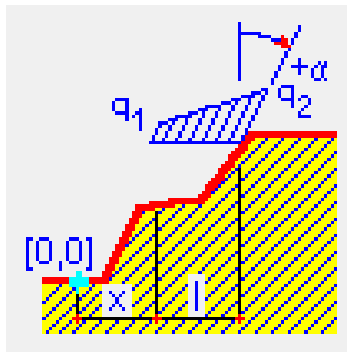
- Force, displacement, stress, strain, pressure
- Stiffness, strength
- 2D, plane strain, axisymmetry
- Boundary conditions
- Construction/calculation stages
- First stage, initial geostatic stress
- Groundwater

# Force, loading, surcharge

- Units [kN]

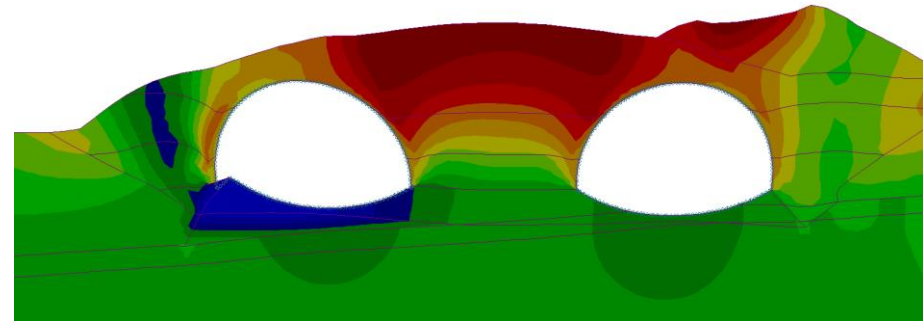
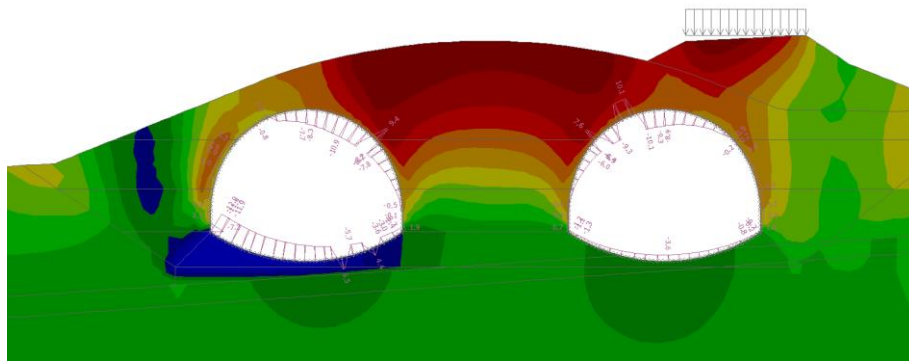


- Force per running, square or cubic meter
- Distributed loading, specific weight, ...
- Units: [kN/m, kN/m<sup>2</sup>, kN/m<sup>3</sup>]



# Displacement

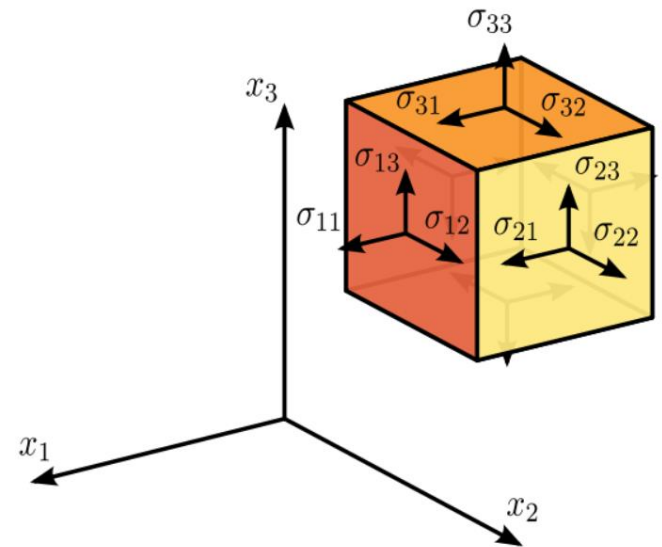
- Change in position
- Vector quantity:  $d_x, d_z$
- Units [mm]
- Vertical displacement = settlement



- I will avoid using the term “deformation”

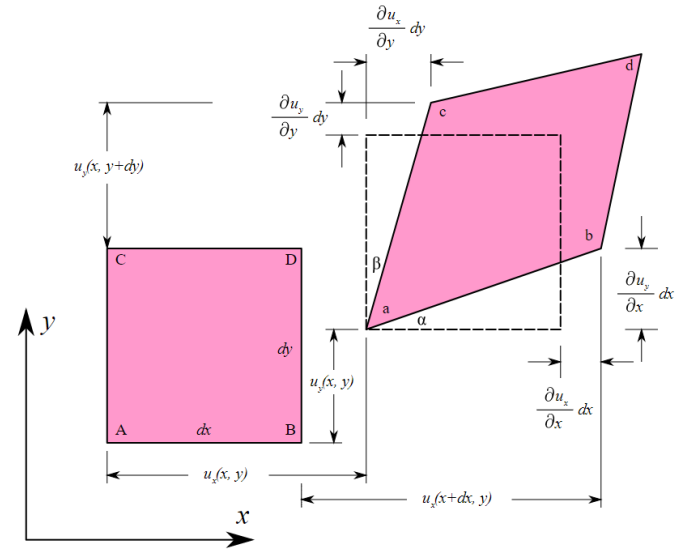
# Stress $\sigma$

- Quantity at material point level
- Units [kPa]
- Tensorial – it has components
  - Normal stress  $\sigma_x, \dots$
  - Shear stress  $\tau_{xz}, \dots$
- Invariants
  - Mean stress  $\sigma_m$
  - Equivalent deviatoric stress  $J$



# Strain $\varepsilon$

- Change in shape and size of small piece of material
- Units [%]
- Tensor quantity
  - Normal strain  $\varepsilon_x, \dots$
  - Shear strain  $\gamma_{xz}, \dots$
- Invariants
  - Volumetric strain  $\varepsilon_v$
  - Equivalent deviatoric strain  $E_d$



# Stiffness

- At material level
  - Hooke's law of elasticity
$$\sigma = D\varepsilon$$
  - Stiffness parameters
    - Young's modulus  $E$  [kPa]
    - Shear modulus  $G$  [kPa]
    - Poisson's ratio  $\nu$  [–]
- Structure with low stiffness will deform excessively
- Stiffness is different from strength



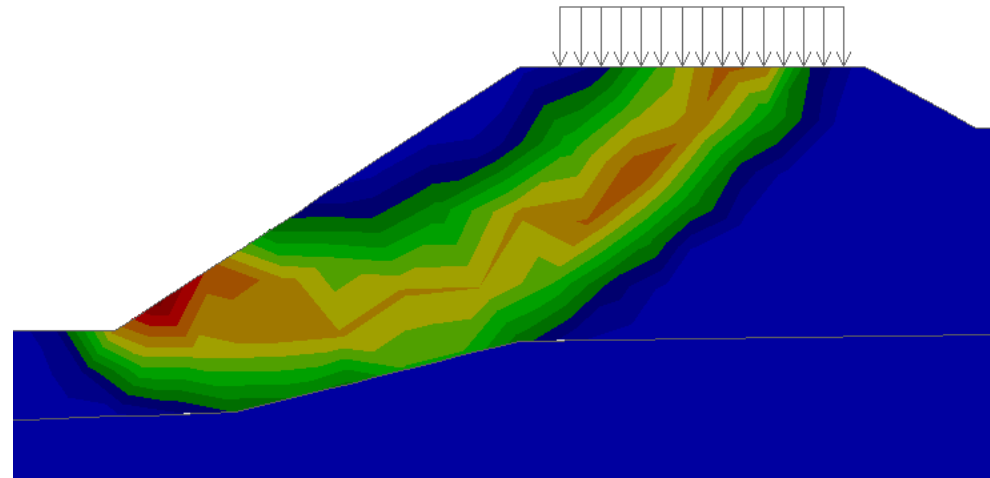


# Strength

- Stress level at failure
- Strength depends on material model
  - Elastic model – infinite strength
  - Mohr-Coulomb:
    - Angle of internal friction  $\varphi$
    - Cohesion  $c$
- Low strength → high plastic strain, unstable structure, failure

# Plasticity

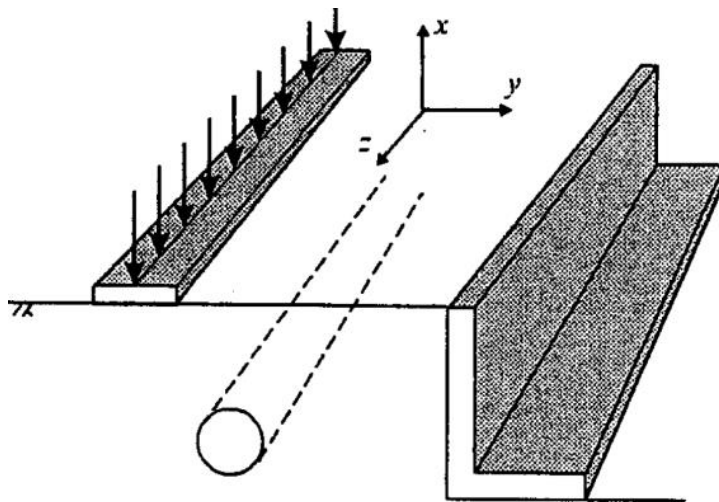
- **Elastic strain** – reversible
  - Deformation of the soil's grains itself
- **Plastic strain** – irreversible
  - Change in arrangement of the soil's grains
- High plastic strains
  - problematic areas
- Equivalent deviatoric plastic strain  $E_{d,pl}$



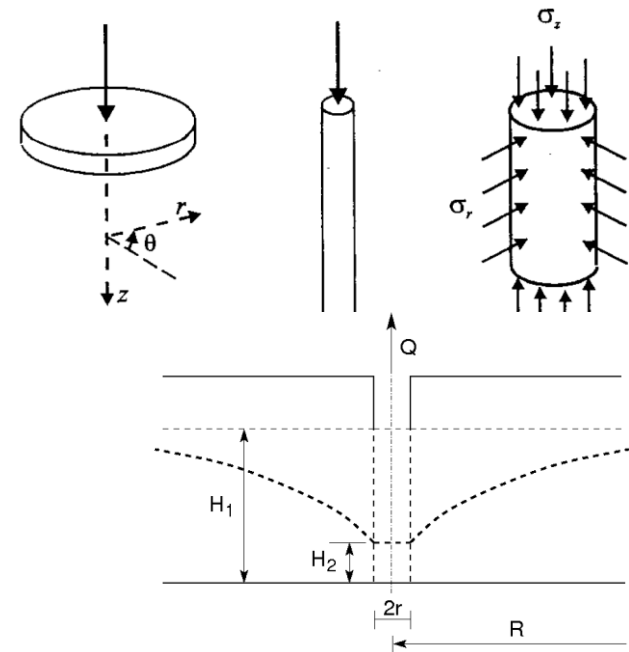
# 2D analysis

- Real world is 3D
- Most real world structures can be represented with 2D model (pile walls, row of anchors, ...)

## Plane-strain

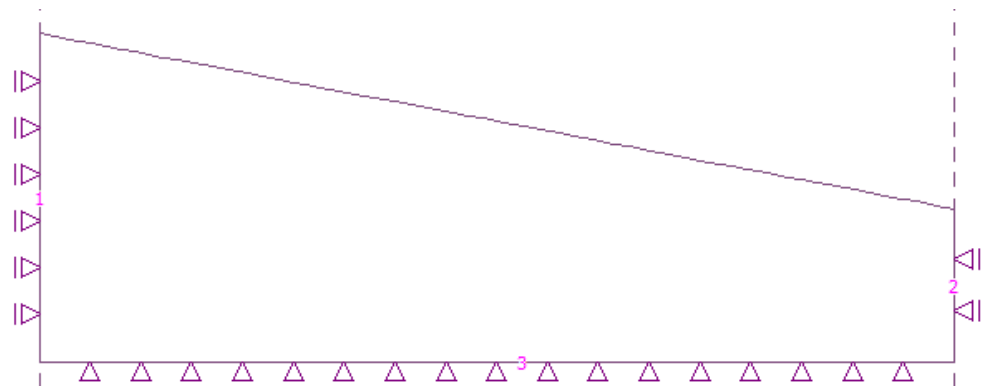
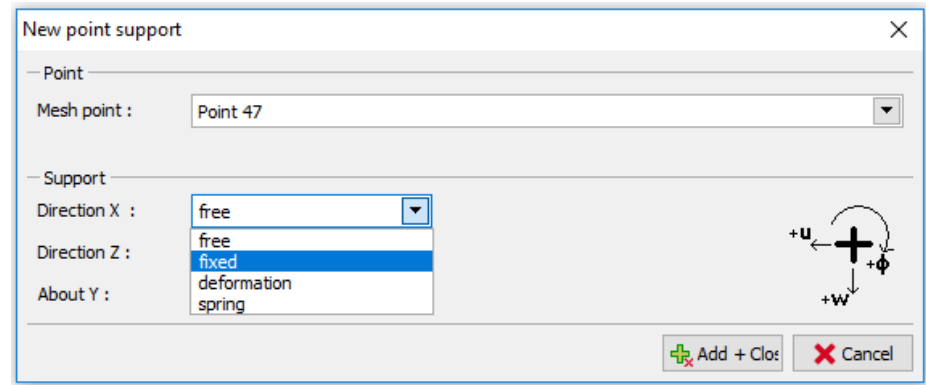


## Axisymmetry



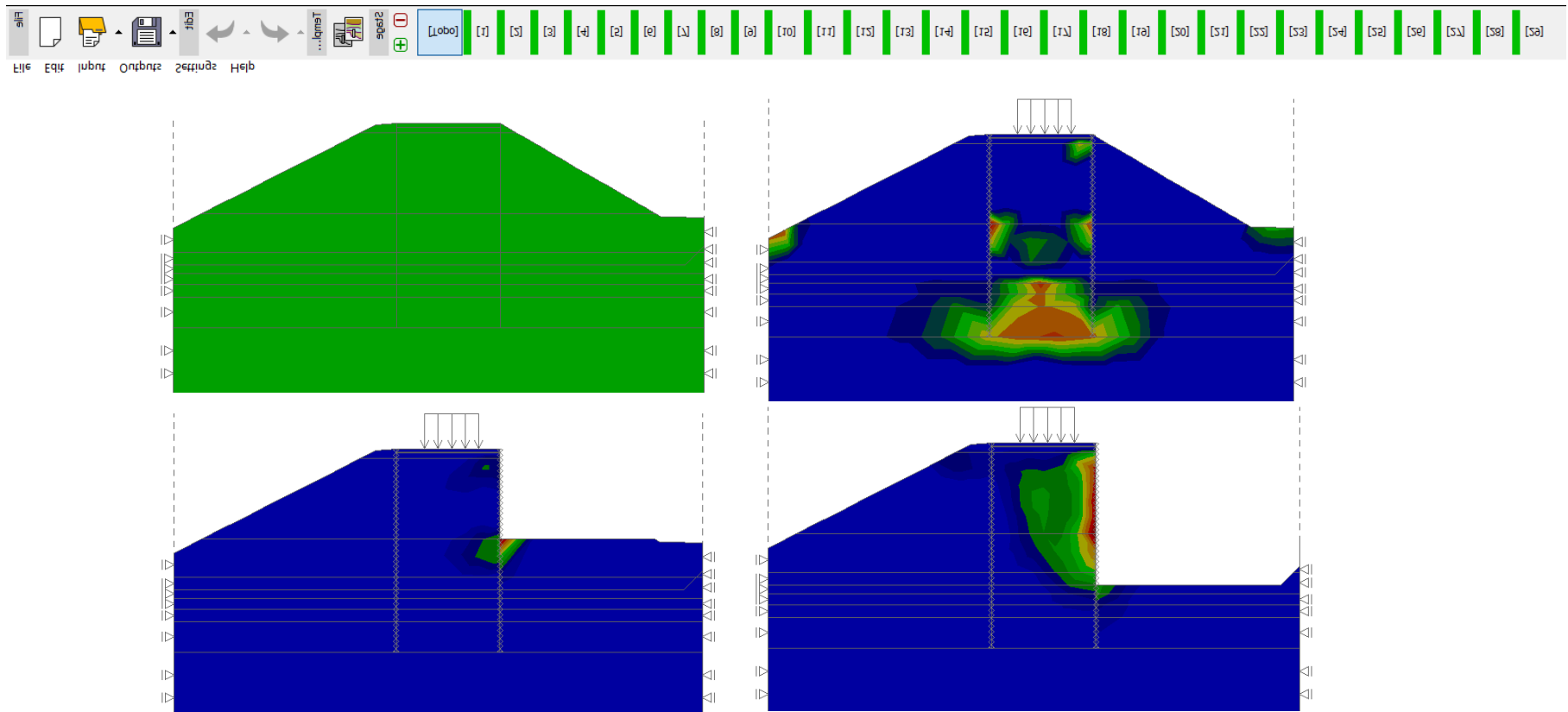
# Boundary conditions

- Prevent or prescribe displacement or rotation
- „Supports“
- **Point supports**
- **Line supports**
  - Added automatically at the bottom and lateral boundaries
  - Might be edited by user



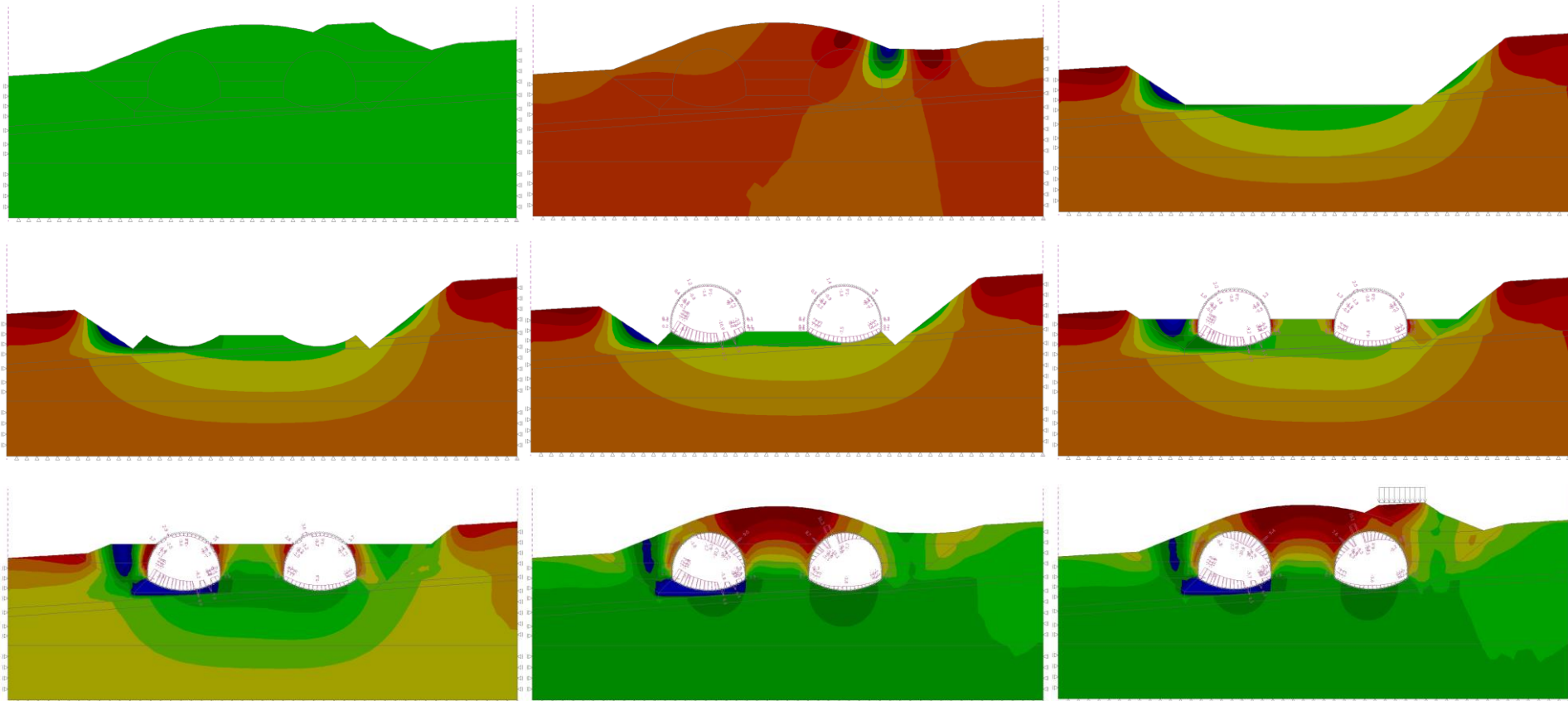
# Construction/calculation stages

- Incremental solution strategy
- Stages are not variants (order matters)



# Construction stages

## Cut and cover tunnel



# First stage

- Special stage to create initial geostatic stress
- Two ways to do that:
  - Load the domain with gravity
  - Get vertical stress from unit weight and horizontal stress from coefficient of lateral pressure  $K_0$

The screenshot shows the 'Settings' window of a software application, divided into three main sections:

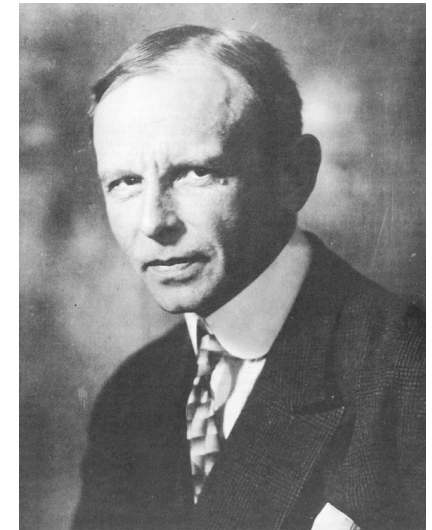
- Project parameters:** Project type is set to 'Plane strain', Analysis type is 'Stress', 'Tunnels' is checked, and 'Allow to input water as the result of steady state water flow analysis' is unchecked.
- Design standards:** Concrete structures is set to 'EN 1992-1-1 (EC2)'. Under 'Calculation of geostatic stress (1st stage)', the Analysis method is set to 'Geostatic stress'.
- Advanced program options:** All options are unchecked: 'Advanced mesh generating parameters', 'Advanced soil parameters', 'Advanced soil models', 'Temperature load', and 'Detailed results'.

- Displacement is zeroed in first stage (undeformed state)

# Groundwater

- **Karl von Terzaghi (1883–1963)**

Austrian civil and geotechnical engineer known as the "father of soil mechanics" born in Prague



- Terzaghi theory of effective stresses

$$\sigma = \sigma' + u$$

- All observable changes in soil are result on change in *effective* stress



# Groundwater

- No water
- Prescribed GWT
- Detailed pore pressure field
  
- Materials
  - Drained – permeable soil, pore pressure excess will dissipate
  - Undrained pore pressure excess is calculated, no volumetric strain

# Golden rule of FEM modeling

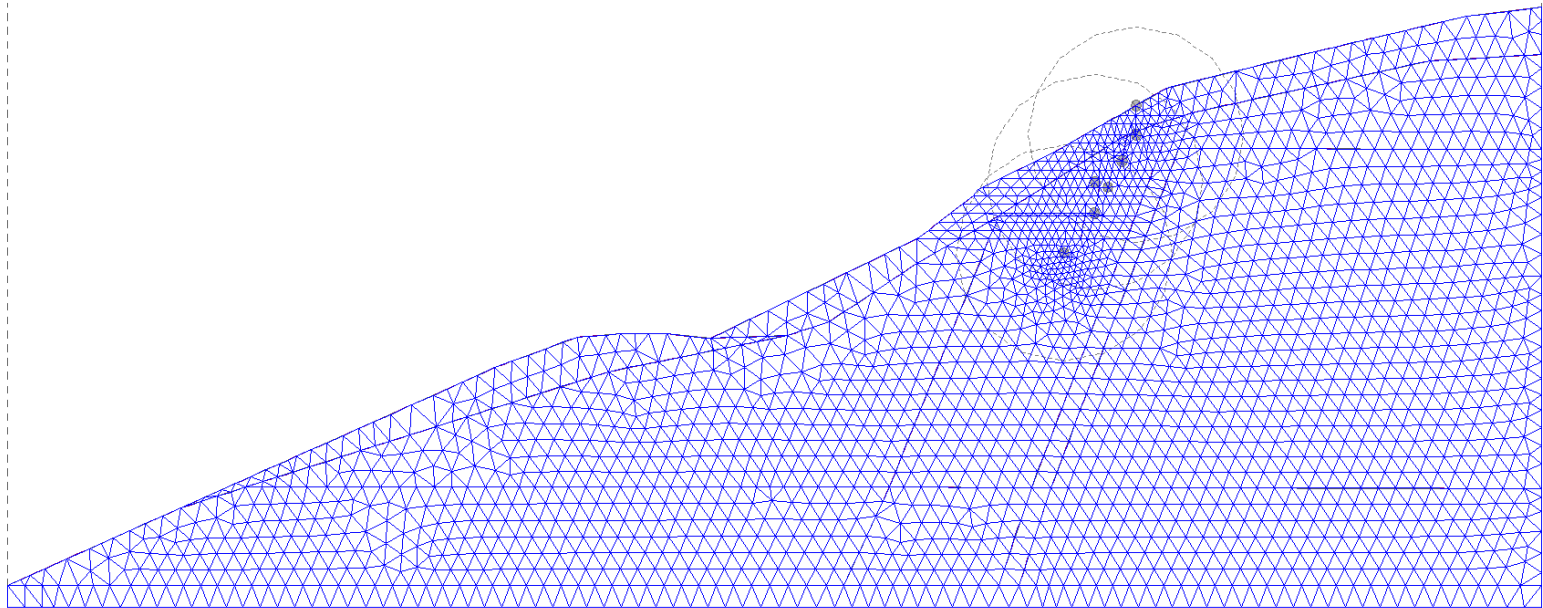
**Make your model as simple as possible,  
but not simpler**

(3D → 2D, simple topology, material model)

*Everything should be made as simple as possible,  
but not simpler—Albert Einstein*

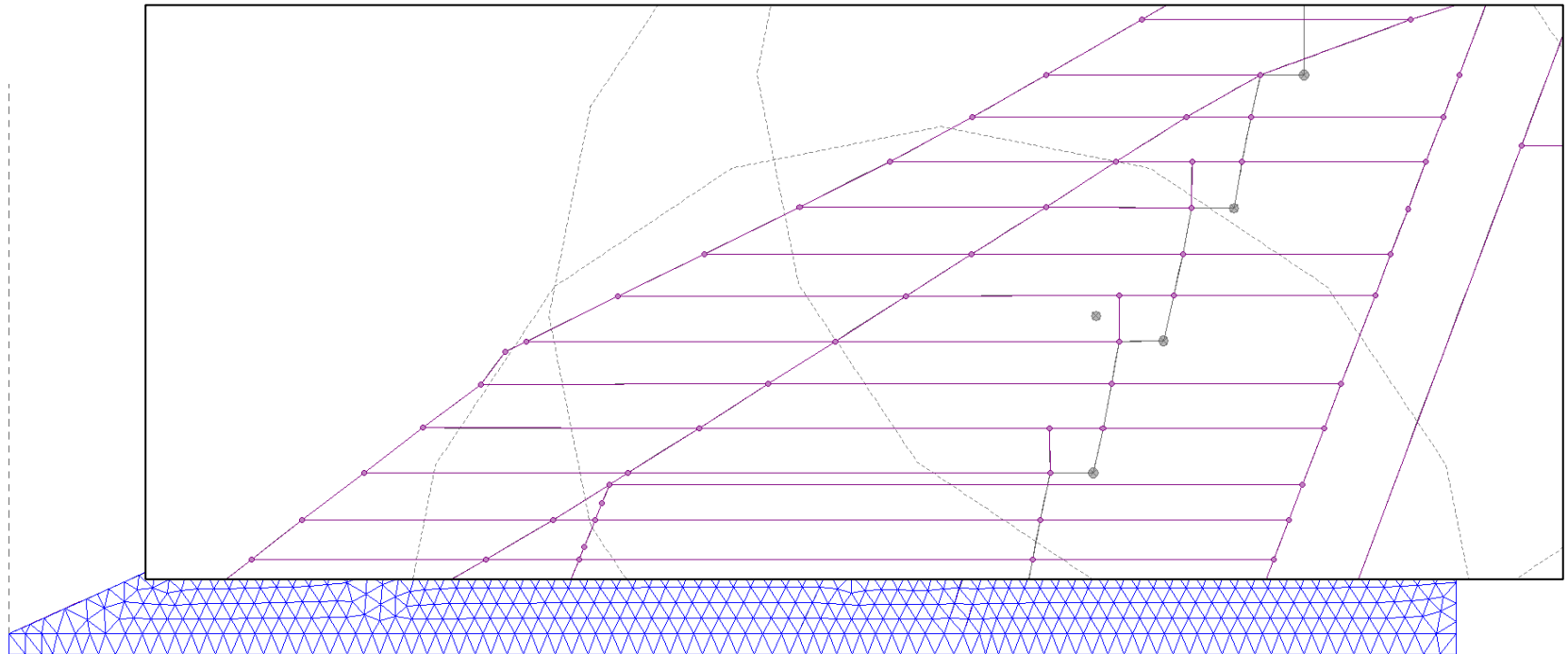
# FE Mesh

- Avoid pointy triangles
  - Refine mesh adequately
  - Modify topology



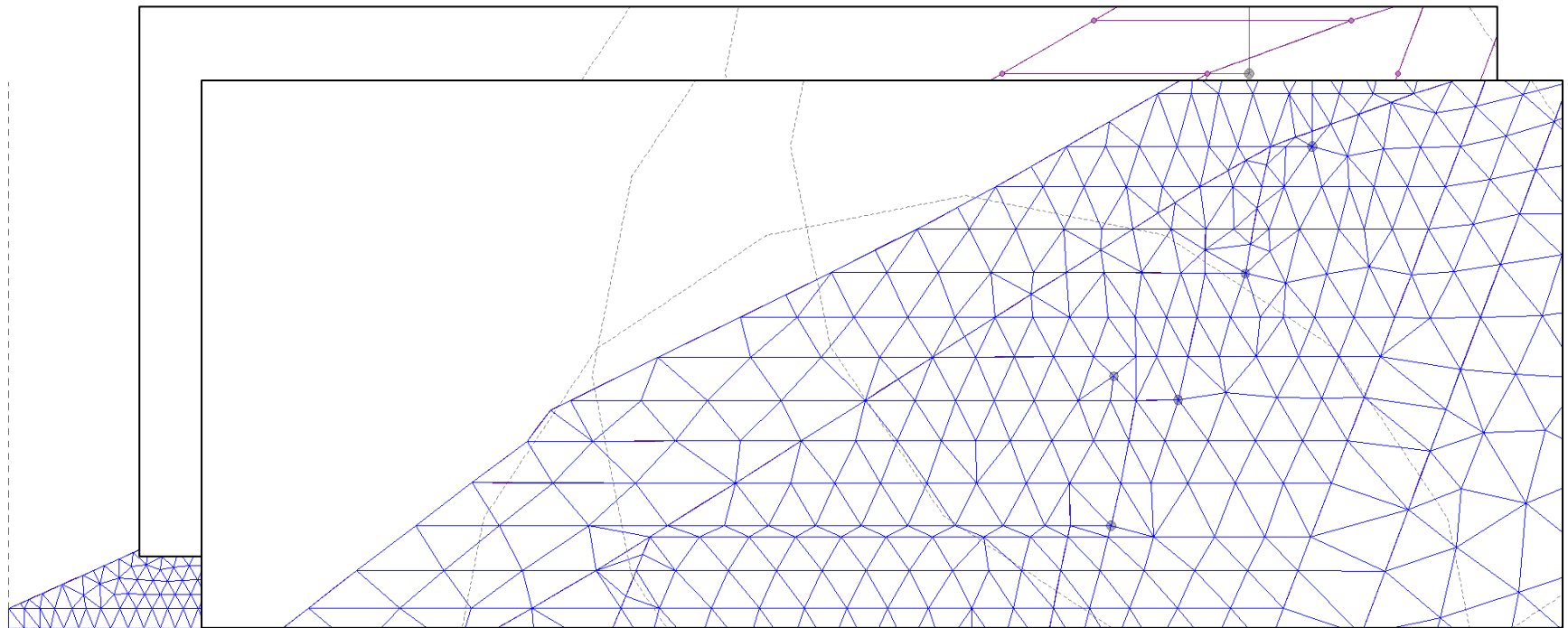
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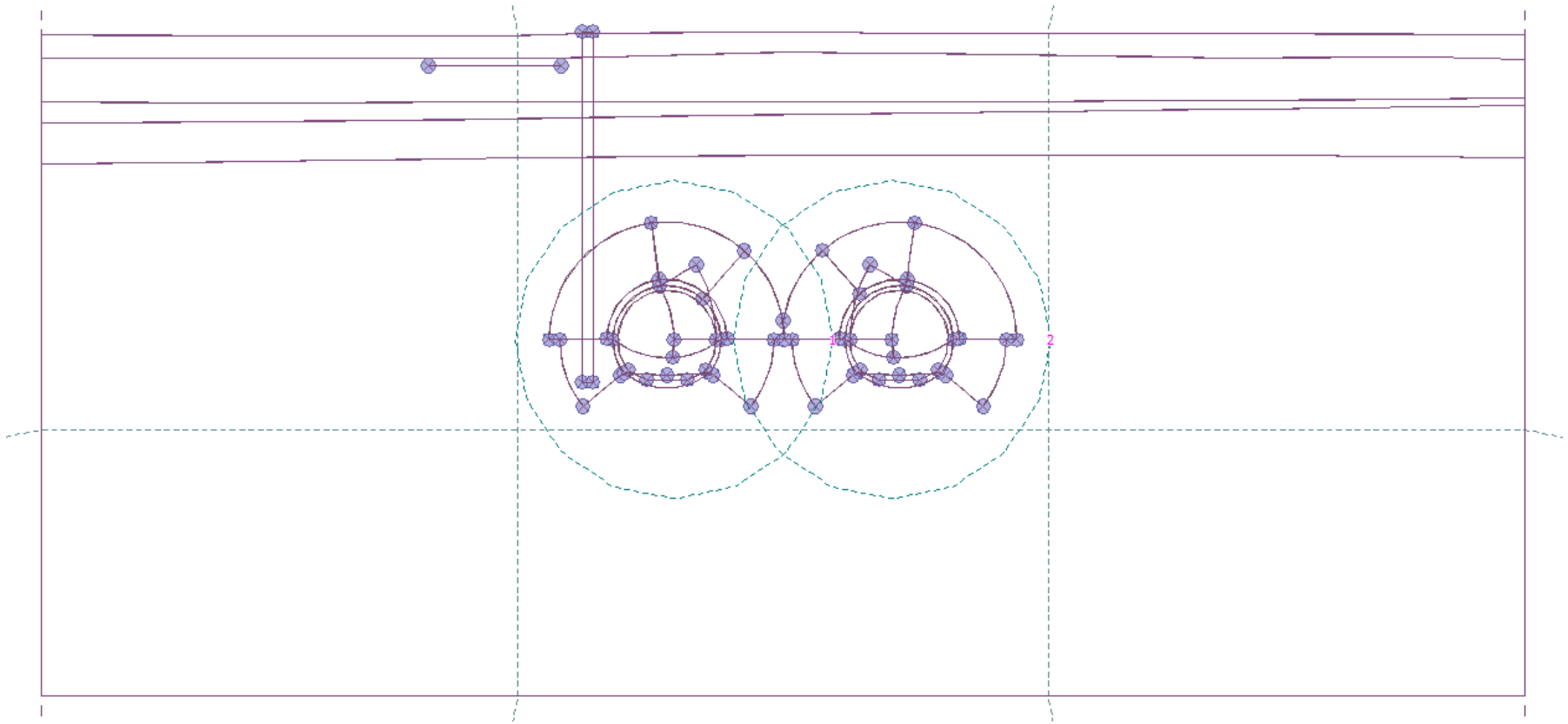
# FE Mesh

- Avoid pointy triangles
  - Refine mesh adequately
  - Modify topology



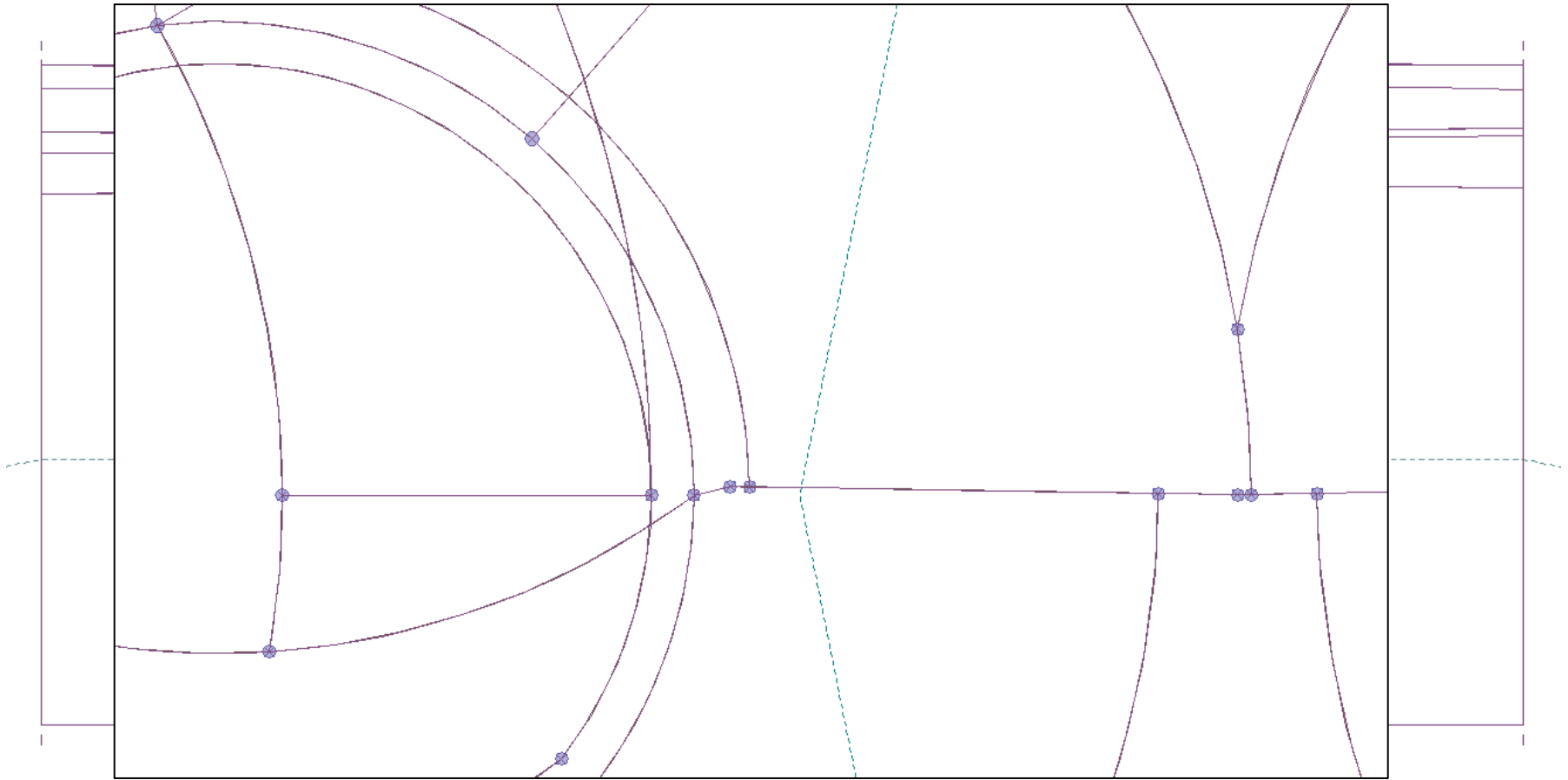
# FE Mesh

- Overly complicated topology



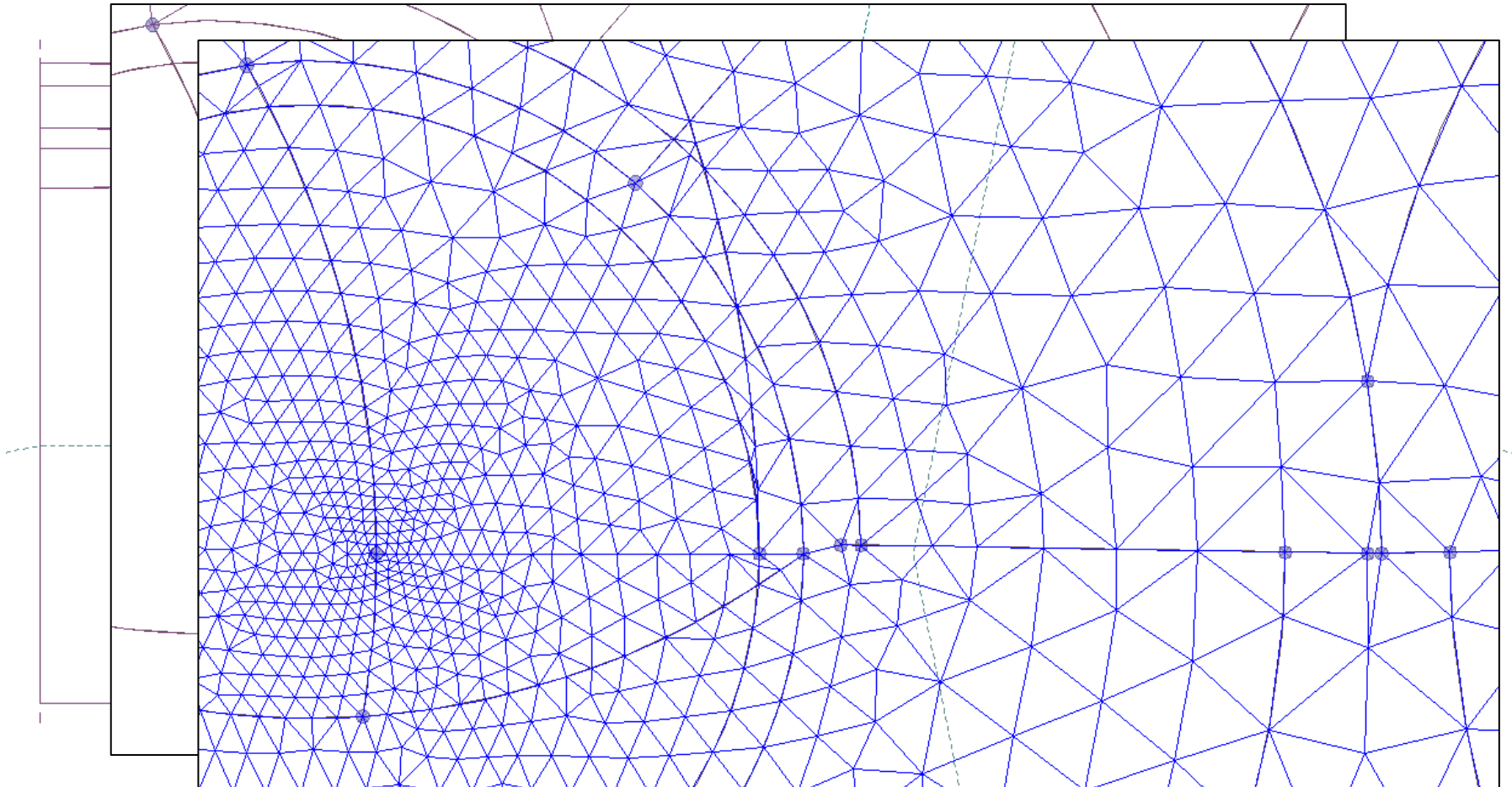
# FE Mesh

- Overly complicated topology



# FE Mesh

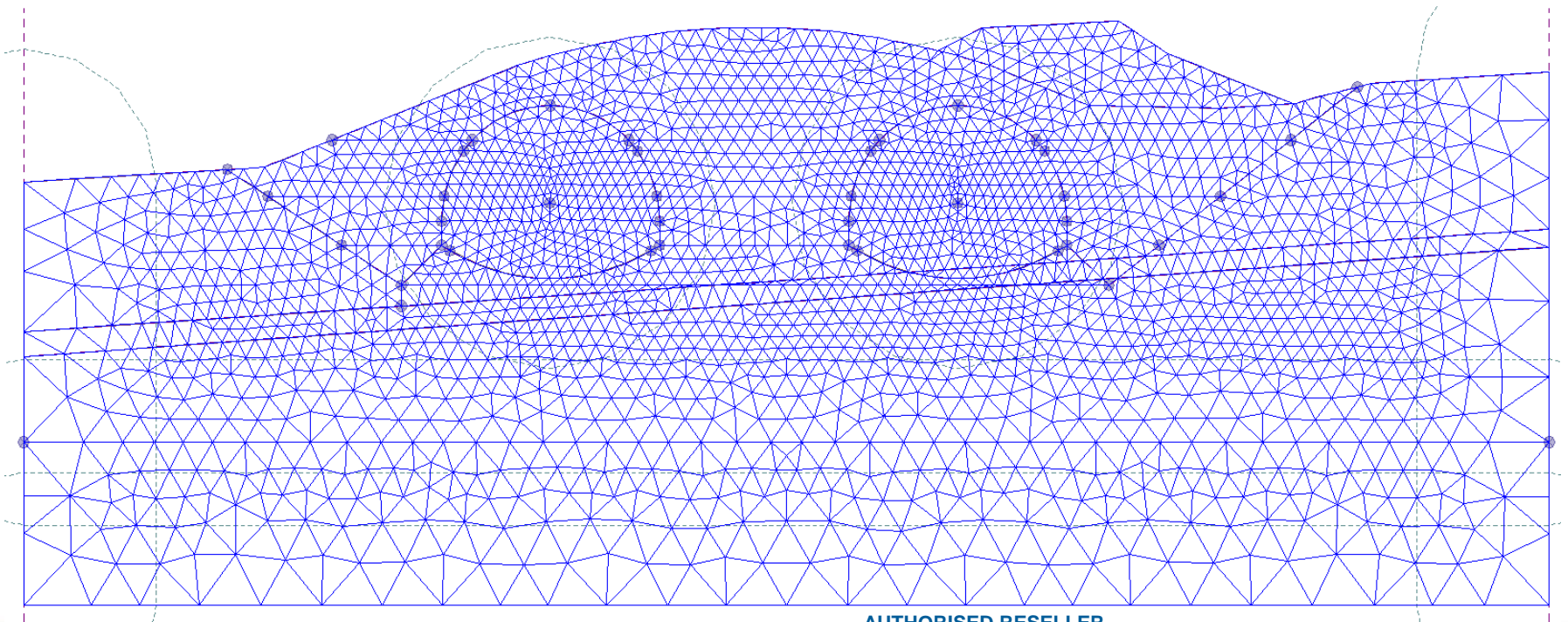
- Overly complicated topology





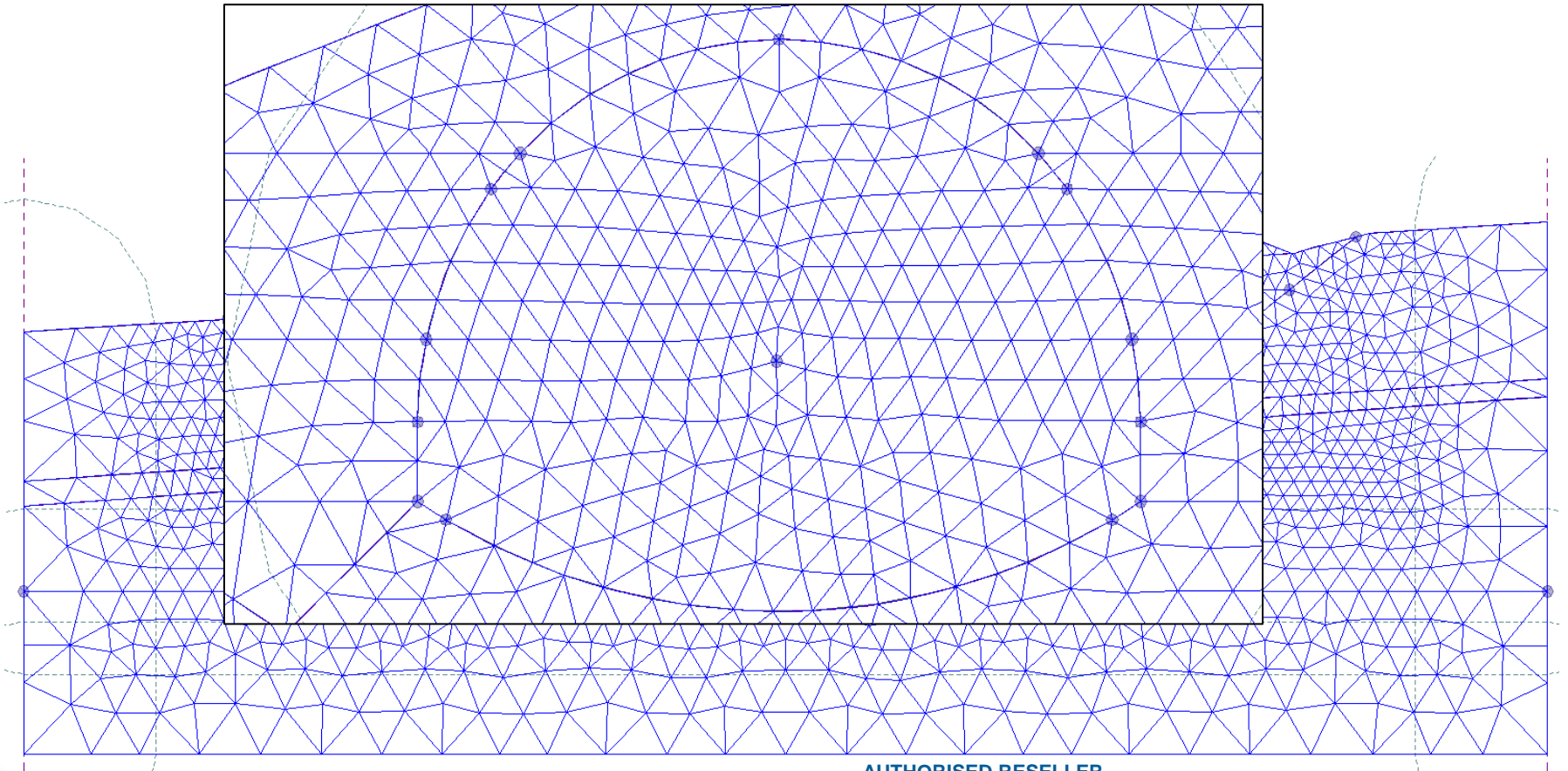
# FE mesh

- This mesh looks OK



# FE mesh

- This mesh looks OK



# Stability analysis

- **Strength reduction method**

Analysis converge → equilibrium → stable structure

No convergence → unstable structure

- Strength parameters  $\varphi$  and  $c$  are decreased to get unstable state

- Factor of safety

$$FS = \frac{\varphi_s}{\varphi_p}$$

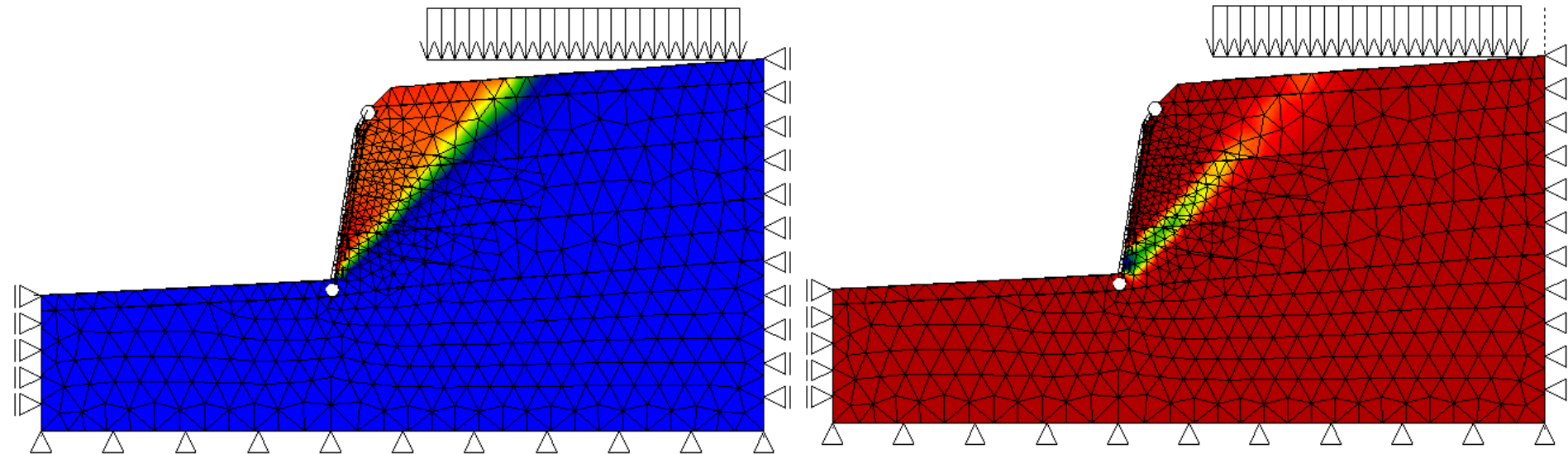
$\varphi_s$  ... real angle of internal friction

$\varphi_p$  ... reduced angle of internal friction at failure

# Stability analysis

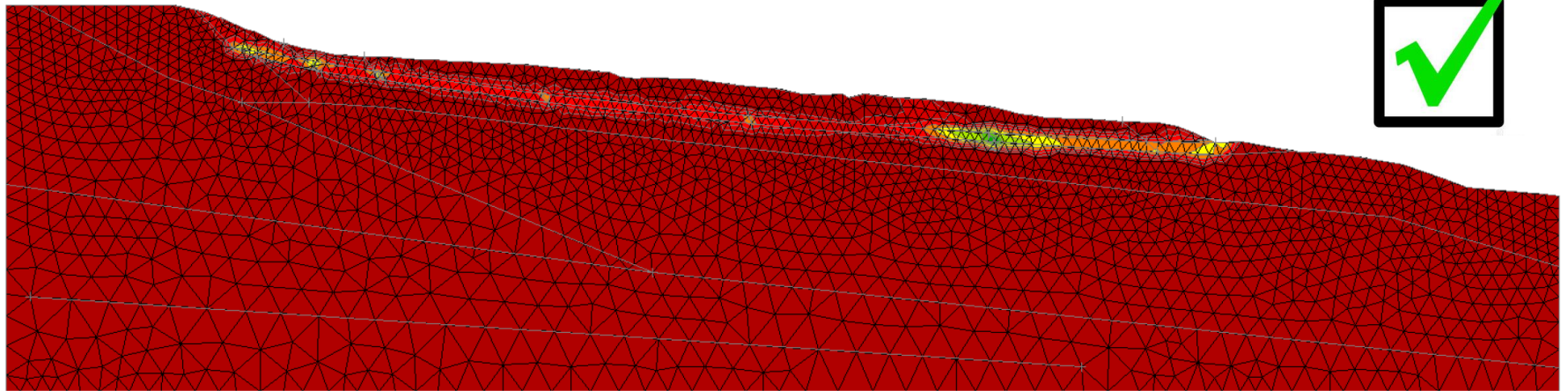
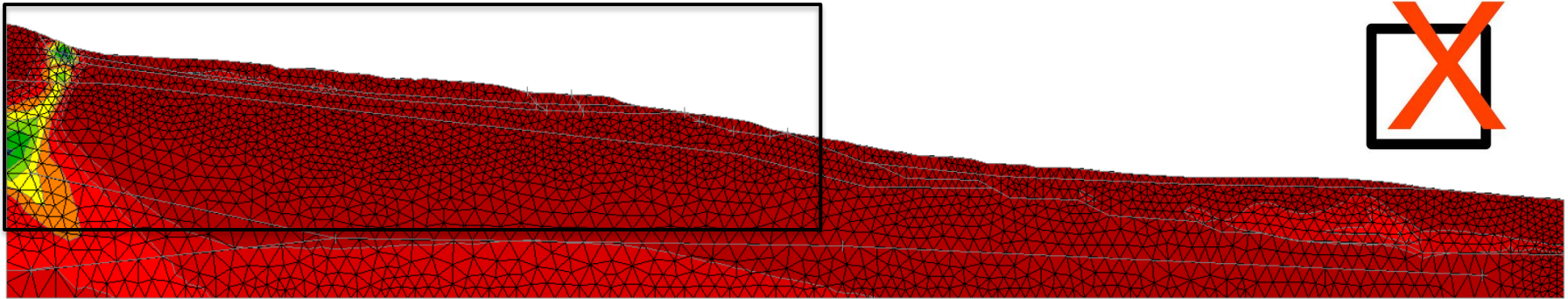
- Results

- Examine the equivalent deviatoric plastic strain  $E_{d,pl}$
- Shear band = slip surface
- Not clear failure mechanism → not reliable  $FS$



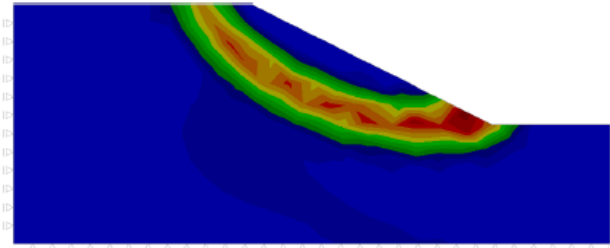
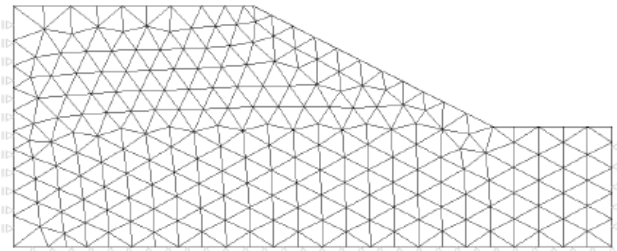
# Stability

- Not clear failure mechanism → not reliable  $FS$
- Distribution of  $E_{d,pl}$

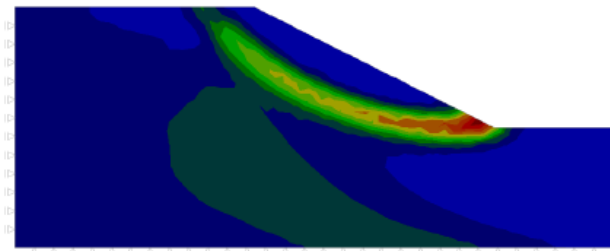
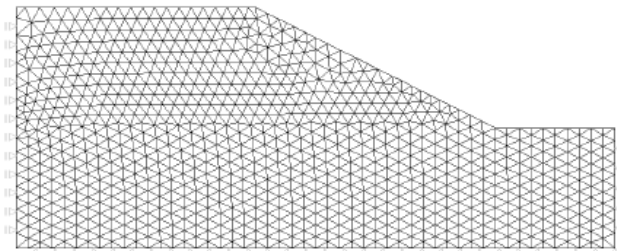


# Stability

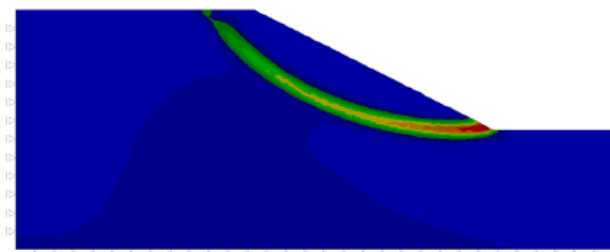
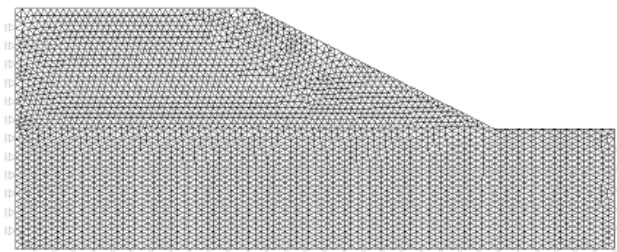
- Finer mesh
  - slightly smaller  $FS$
  - slightly narrower plastic zone



FS=1.50



FS=1.46



FS=1.44



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Assistenza Remota